

VRARIA SELBACH

— DE —
SELBACH & CIA.

Successores de J. R. DA FONSECA & CIA.

Rua Marechal Floriano, 10

Caixa Postal 52 - Telefones 4394 e 4694

Endereço telegr. e phonogr.: "SELBACH"

PORTO ALEGRE

Nº 24629

Nº Leg

Gr

S-83

De 4 plantas e
e referências em defesa?

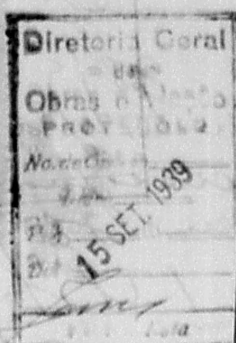
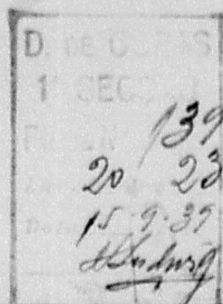
SELBACH & CIA., proprietários da "Divisão Selbach",

estabelecida à Rua Marechal Floriano n.º 10, nesta capital, vêm
pedir a V.S. se digna conceder-lhes licença para a construção
de nova fachada no prédio de sua propriedade, sito à Rua Mare-
chal Floriano n.º 10, bem como para a reforma interior, tudo de
conformidade com as incluídas plantas.

As referidas obras serão feitas por administração dos
próprios requerentes e o seu custo não será compensado pela au-
tização de respectivo prédio, em caso de desapropriação.

Nestes termos,

Assinatura



30 Setembro de 1939
Assinatura

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80-9-39
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PUBLICADE*

1429 m. 10

A todos. Preci-
sio, por ser o pre-
disposto pelo meu
Plan dentro, não ve-
mos inconveniente
em ser concedido
o que segue

Em 15/9/39

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19-IX-1939

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Procuradoria

28/9

a' Directoria de obras e viac. ad.

Assim termo
em 28.9.1939.

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24628/39

(2)

DECLARAÇÃO

Para os devidos fins e efeitos declaramos
que, a construção e demais reformas internas, do prédio
de nossa propriedade, sito á Rua Marecha l Floriano N° 10,
será executado pelo constructor Sr. CARLOS JUNG, residente
nesta cidade, á Rua Gaspar Martins N° 345.

PORTO ALEGRE, 11 de Setembro de 1939.

OS PROPRIETARIOS Servando

O CONSTRUCTOR Carlos Jung

Recebo e assos firma e rubrica

Em testemunho

Porto Alegre, 11 de Setembro de 1939.

Assinada do notário em pleno exercício.

Assinada do proprietário

Assinada do construtor

Assinada do notário

Assinada do proprietário

Assinada do construtor

Assinada do notário

Assinada do proprietário

Assinada do construtor



7x500

11937

5 PE/82245
CARLOS JUNG

CONSTRUÇÕES EM GERAL
REFORMAS

RUA GASPAR MARTINS N.º 428
TELEPHONE AUT. 2962

PORTO ALEGRE, 25 de Setembro de 1939.

ORÇAMENTO para os Srs. SELBACH & CIA.

N/Capital.

CARLOS JUNG, construtor, residente nesta cidade,
à Rua Gaspar Martins Nº 345, propõe-se a executar a construção
de uma nova fachada, bem como outros pequenos reparos internos,
do prédio, situado à Rua Marechal Floriano Peixoto Nº 10, e de
propriedade dos Srs. SELBACH & CIA. tudo de conformidade com
a planta, pelo preço total de TRINTA E CINCO CONTOS DUZENTOS E
QUARENTA E TREIS MIL REIS - R\$: 35:243\$000.- - - - - -

PORTO ALEGRE, 25 DE SETEMBRO DE 1939.

Carlos Jung





PREFEITURA MUNICIPAL DE PORTO ALEGRE
SMA - SERVIÇO DE DOCUMENTAÇÃO
SEÇÃO DE MICROFILMAGEM

AS PLANTAS E DESENHOS
QUE ULTRAPASSARAM A MEDI-
DA PADRÃO, CONSTANTES DÊS
TE PROCESSO, ESTÃO MICRO
FILMADAS NO FILME DE 35mm :

24628/39

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FLASH 02	DATA 24.10.45	
ARQ 001	GAV 02	BOX 03
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FROM THE DEPARTMENT OF
THE SECRETARY OF THE
NAVY
WASHINGTON, D. C.

NAVY DEPARTMENT
WASHINGTON, D. C.

$$M_1 = \frac{1}{18} 1100 \times 2.15 = +159$$

$$X_1 = \frac{1}{12} 1100 \times 2.15 = -197$$

$$X_2 = \frac{1}{2} 1100 \times 2.15 = -99$$

$$M_2 = \frac{1}{18} 1100 \times 2.15 = +159$$

Engaste A $\sigma b = 18$ d. 20 $T_0 = 0.5$

Centro 1 $\sigma b = 18$ d. 20 $T_0 = 0.82$

Engaste B $\sigma b = 18$ d. 20 $T_0 = 1.04$

Centro 2 $\sigma b = 18$ d. 20 $T_0 = 0.67$

Estribos $\square \phi 3/8$, c. 10 cms.

Consolas C.1.C.2.C.3.C.4 Vão 1.60

Carga conc. 1.100 $M = 1100 \times 1.60 = 1760$

Carga uniforme, laje 1.80 x 1.55 x 310 = 550

P.R. 1.80 x 2.20 x 1.55 x 310 = 960

$M = \frac{1}{2} 1760 \times 1.60 = 1408$

$M = \frac{1}{2} 960 \times 1.60 = 768$

$M = 1408 + 768 = 2176$

$b_0 = 18$ $\sigma b = 4.7$ h. 42 d. 45 $T_0 = 5.5$ $\phi 3/8$

Est. $\square 1/4$ c. 15 cm. $T_0 = 3.00$

Viga da Marquisa

Cargas uniformes = 510

Ferro 510 x 0.80 x 1.50 = 1870

Laje 5.25 x 0.80 x 1.50 = 2040

P.R. 0.85 x 0.85 x 1.50 x 1.50 = 3220

$M = \frac{1}{12} 4000 \times 5.50 = 1850$

$X = \frac{1}{24} 4000 \times 5.50 = 930 \sim 1000$

$P_1 P_2 = 1100 \times 960 = 1960 \sim 2000$

$M = \frac{1}{2} \times \frac{2000 \times 1.55}{5.25} = 1530 \sim 1550$

$X = - \frac{2000 \times 1.55 \times 0.7}{5.25} = 1090 \sim 1100$

$M = \frac{1550}{2100} \times -1100 = -800$

Centro $\sigma b = 30.15$ h. 60 d. 65 $T_0 = 5.20$ $\phi 3/8$

Engaste 22.0 " " $T_0 = 9.16$ $\phi 3/8$

$G_0 = \frac{4000}{0.85 \times 2.5 \times 1.80} = 3.03$ Est. $\square \phi 1/4$ c. 20

Porto Alegre, 7.9.1939.

Impr. Carlos Zing

Continuação P.3



de concreto

Carga 450

$0.50 \times 0.85 \times 1.60 = 0.70$

$M = \frac{1}{2} 450 \times 5.50 = 1237.5$

$X = \frac{1}{24} 450 \times 5.50 = 103.1$

$M = 1237.5 + 103.1 = 1340.6$

$b_0 = 10$

Vigas V3 e V2 - Vão: 5.50

$M = \frac{1}{2} 5480 \times 5.50 = 15080$

$X = \frac{1}{24} 5480 \times 5.50 = 1260$

$x = 17.35$ Centro: $\sigma b = 47$ h. 47 d. 50 $T_0 = 5.10$ $\phi 3/8$

Engaste: $\sigma b = 34.5$ h. 47 d. 50 $T_0 = 2.47$ $\phi 3/8$

$V = 27.40$ $T_0 = \frac{27.40}{0.85 \times 1.5 \times 4.7} = 4.42$ $\square 3/8$ c. 15 cm.

Laje consola igual a laje consola da viga V.4

Marquisa: Vão, balanço: 1.60

$2.15 + 2.15 + 2.15 = 6.45$

Laje: a b a

$2.15 + 2.15 + 2.15 = 6.45$

$M_0 = \frac{1}{12} 320 \times 2.15 = +195$

$M_0 = \frac{1}{12} 320 \times 2.15 = +99$

$X_1 = \frac{1}{18} 320 \times 2.15 = -22$

$X_2 = \frac{1}{18} 320 \times 2.15 = -165$

Engaste: $\sigma b = 24.7$ h. 5.5 d. 7.0 $T_0 = 1.83$

Centro: $\sigma b = 33.5$ h. 5.5 d. 7.0 $T_0 = 2.27$

Engaste: $\sigma b = 20$ h. 5.5 d. 7.0 $T_0 = 1.65$

Centro: $\sigma b = 30$ h. 5.5 d. 7.0 $T_0 = 2.81$

Distribuição: $\phi 3/8$ R.H.

Vigas: T.1 e T.2 - Vão 2.15

Laje 0.50 x 2.20 x 2.15 = 2.34

P.P. 0.80 x 2.15 x 2.15 = 3.66

Ferro 0.80 x 2.15 x 2.15 = 3.66

$M = 2.34 + 3.66 = 6.00$

$X = \frac{1}{24} 6.00 \times 2.15 = 0.54$

$M = 6.00 + 0.54 = 6.54$

$M = 6.54 + 0.54 = 7.08$

$M = 7.08 + 0.54 = 7.62$

$M = 7.62 + 0.54 = 8.16$

$M = 8.16 + 0.54 = 8.70$

$M = 8.70 + 0.54 = 9.24$

$M = 9.24 + 0.54 = 9.78$

Continuação P.2

Parede 0.20 x 0.15 x 1.60 = 0.48

P.R. 0.20 x 0.15 x 1.60 x 2.400 = 7.2

$M = \frac{1}{2} 120 \times 0.2 = 12$

$G_0 = 2.0$ h. 7.5 d. 10 $T_0 = 1.23$

ϕ da distribuição: $2 \phi 3/8$ $\phi 7/16$ p. M.

Cargas uniformes

Parede 0.50 x 0.80 x 2.25 x 1.60 = 12.60

P.P. 5.25 x 0.80 x 2.25 = 23.60

P.R. 5.25 x 0.80 x 2.25 x 2.400 = 19.20

Parede 0.30 x 0.85 x 2.25 x 1.60 = 8.60

$M = 12.60 + 23.60 + 19.20 + 8.60 = 64.00$

$X = \frac{1}{24} 64.00 \times 5.50 = 149.3$

$M = 64.00 + 149.3 = 213.3$

$M = 213.3 + 149.3 = 362.6$

$M = 362.6 + 149.3 = 511.9$

$M = 511.9 + 149.3 = 661.2$

$M = 661.2 + 149.3 = 810.5$

$M = 810.5 + 149.3 = 959.8$

$M = 959.8 + 149.3 = 1109.1$

$M = 1109.1 + 149.3 = 1258.4$

$M = 1258.4 + 149.3 = 1407.7$

$M = 1407.7 + 149.3 = 1557.0$

$M = 1557.0 + 149.3 = 1706.3$

$M = 1706.3 + 149.3 = 1855.6$

$M = 1855.6 + 149.3 = 2004.9$

$M = 2004.9 + 149.3 = 2154.2$

$M = 2154.2 + 149.3 = 2303.5$

$M = 2303.5 + 149.3 = 2452.8$

$M = 2452.8 + 149.3 = 2602.1$

$M = 2602.1 + 149.3 = 2751.4$

$M = 2751.4 + 149.3 = 2900.7$

$M = 2900.7 + 149.3 = 3050.0$

$M = 3050.0 + 149.3 = 3199.3$

$M = 3199.3 + 149.3 = 3348.6$

$M = 3348.6 + 149.3 = 3497.9$

$M = 3497.9 + 149.3 = 3647.2$

$M = 3647.2 + 149.3 = 3796.5$

$M = 3796.5 + 149.3 = 3945.8$

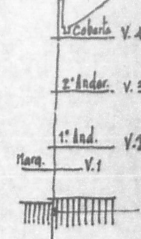
$M = 3945.8 + 149.3 = 4095.1$

$M = 4095.1 + 149.3 = 4244.4$

$M = 4244.4 + 149.3 = 4393.7$

LIVRARIA SELBACH - Cálculos de Cimento da Fachada asar Cam a Rua Hial. Floriano,

Colunas



3.º Per. tomado $7.05 \times 1.75 \times 120 = 1260$

Piso $0.25 \times 3.55 \times 800 = 690$

Parede $1.50 \times 3.55 \times 400 = 210$

Viga $0.25 \times 0.25 \times 4.35 \times 1600 = 350$

$M = 1260 + 690 + 210 + 350 = 2510$

2.º Per. Piso $0.55 \times 3.55 \times 800 = 1500$

Parede $3.55 \times 1.00 \times 400 = 1420$

Viga $3.55 \times 0.30 \times 1.00 \times 2400 = 2556$

$M = 1500 + 1420 + 2556 = 5476$

1.º Per. Pedestal $0.30 \times 0.30 \times 2400 = 2160$

Terra $0.25 \times 0.25 \times 1.00 \times 2400 = 600$

Sapata $0.25 \times 0.25 \times 0.50 \times 2400 = 300$

$M = 2160 + 600 + 300 = 3060$

Sapata

$\frac{3060}{4.5} = 680$

$\sqrt{5900} = 77$

$\frac{30}{77} = 0.39$

$k = 0.21$

$\frac{20050}{30 \times 85} = 12$

$f = 22 + 5 = 27$

$M = 0.37 \times 2665 \times 0.85 = 835$

$F_0 = \frac{83500}{5.2 \times 0.7 \times 1200} = 3.50$

$\phi 3/8$

Viga V.4 - Vão: 5.25 x 2.5 = 5.50

Carga concentrada

$P_1 = 3.75 \times 3.15 \times 120 = 900$

$P_2 = 9.00 \times 5.50 = +1020$

$X = \frac{1}{16} 900 \times 5.50 = -310$

$M = -310 + 1020 = 710$

$M = 710 + 1020 = 1730$

$M = 1730 + 1020 = 2750$

$M = 2750 + 1020 = 3770$

$M = 3770 + 1020 = 4790$

$M = 4790 + 1020 = 5810$

$M = 5810 + 1020 = 6830$

$M = 6830 + 1020 = 7850$

$M = 7850 + 1020 = 8870$

$M = 8870 + 1020 = 9890$

$M = 9890 + 1020 = 10910$

$M = 10910 + 1020 = 11930$

$M = 11930 + 1020 = 12950$

Cargas uniformes

Parede 5.50 x 1.10 x 1.60 = 9.68

Piso 5.50 x 0.80 x 1.60 = 7.04

P.R. 5.50 x 0.80 x 1.60 x 2.400 = 26.60

$M = 9.68 + 7.04 + 26.60 = 43.32$

$X = \frac{1}{24} 43.32 \times 5.50 = 10.0$

$M = 43.32 + 10.0 = 53.32$

$M = 53.32 + 10.0 = 63.32$

$M = 63.32 + 10.0 = 73.32$

$M = 73.32 + 10.0 = 83.32$

$M = 83.32 + 10.0 = 93.32$

$M = 93.32 + 10.0 = 103.32$

$M = 103.32 + 10.0 = 113.32$

$M = 113.32 + 10.0 = 123.32$

$M = 123.32 + 10.0 = 133.32$

$M = 133.32 + 10.0 = 143.32$

$M = 143.32 + 10.0 = 153.32$

$M = 153.32 + 10.0 = 163.32$

$M = 163.32 + 10.0 = 173.32$

Continua

$$\begin{aligned} \frac{1}{18} 1100 \times 2.15 &= +159 \\ \frac{1}{12} 1100 \times 2.15 &= -197 \\ \frac{1}{2} 1100 \times 2.15 &= -99 \\ \frac{1}{18} 1100 \times 2.15 &= +131 \end{aligned}$$

$$\begin{aligned} \text{gasto A } 3 \text{ } \phi 18 \text{ d. } 20 \text{ Fe } &= 0.5 \\ \text{gasto B } 3 \text{ } \phi 18 \text{ d. } 20 \text{ Fe } &= 0.82 \\ \text{gasto C } 3 \text{ } \phi 18 \text{ d. } 20 \text{ Fe } &= 1.04 \\ \text{gasto D } 3 \text{ } \phi 18 \text{ d. } 20 \text{ Fe } &= 0.87 \end{aligned}$$

Estribos $\square \phi 3/8$, e = 10 cm.

solos C.1.C.2.C.3.C.4 Vão 1.60

$$\begin{aligned} \text{Carga uniforme, laje } 1.60 \times 1.55 \times 3.50 &= 550 \\ \text{P.R. } 1.60 \times 2.20 \times 4.50 \times 2.40 \times 3/10 &= 860 \\ \text{M} = \frac{1260}{2} \times 1.60 &= 1000 \\ \text{M} = \frac{1260}{2} \times 1.60 &= 1000 \end{aligned}$$

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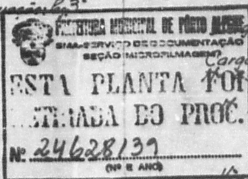
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Continuação p. 3.



Parade 0.20 x 0.15 x 1.00 = 48
P.R. 0.20 x 0.15 x 1.00 x 2.40 = 72
M = $\frac{120}{2} \times 0.2 = 12$
G.b. 20 h. 7.5 d. 10 Fe = 1.23
§ de distribuição: 2 § 3/8

Vigas V3 e V2 - Vão: 5.50

$$\begin{aligned} \text{M} = \frac{1}{12} 5480 \times 5.50 &= 2520 \\ \text{X} = \frac{1}{12} 5480 \times 5.50 &= 1260 \end{aligned}$$

$$\text{X} = 17.35 \text{ Centro: } G.b. 47 h. 47 d. 50 \text{ Fe} = 5.10 \text{ (} \phi 1/2 \text{) } b = 15$$

$$\text{gasto: } G.b. 3as h. 47 d. 50 \text{ Fe} = 2.47 \text{ (} \phi 1/2 \text{) } 9$$

$$\text{V} = 2740 \text{ To} = \frac{2740}{0.88 \times 15 \times 4} = 4.42 \text{ } \square 3/8 \text{ e} = 15 \text{ cm.}$$

Laje consolo igual a laje consolo da viga V4

Marquisa: Vão, balanço: 1.60

$$\begin{aligned} \text{M} = \frac{1}{12} 320 \times 2.15 &= +195 \\ \text{M} = \frac{1}{12} 320 \times 2.15 &= +99 \\ \text{X} = \frac{1}{12} 320 \times 2.15 &= -22 \\ \text{X} = \frac{1}{12} 320 \times 2.15 &= -165 \end{aligned}$$

$$\begin{aligned} \text{M} = \frac{1}{12} 320 \times 2.15 &= +195 \\ \text{M} = \frac{1}{12} 320 \times 2.15 &= +99 \\ \text{X} = \frac{1}{12} 320 \times 2.15 &= -22 \\ \text{X} = \frac{1}{12} 320 \times 2.15 &= -165 \end{aligned}$$

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Continuação p. 2.

$$\begin{aligned} \text{Parade } 0.20 \times 0.15 \times 1.00 &= 48 \\ \text{P.R. } 0.20 \times 0.15 \times 1.00 \times 2.40 &= 72 \end{aligned}$$

$$\text{M} = \frac{120}{2} \times 0.2 = 12$$

$$\begin{aligned} \text{G.b. } 20 \text{ h. } 7.5 \text{ d. } 10 \text{ Fe} &= 1.23 \\ \text{§ de distribuição: } 2 \text{ § } 3/8 & \text{ } \phi 1/4 \text{ p. M.} \end{aligned}$$

$$\begin{aligned} \text{Cargas uniformes} \\ \text{Parade } 0.50 \times 0.30 \times 5.25 \times 1.00 &= 1260 \\ \text{Piso } 5.25 \times 0.80 \times 2.40 &= 2520 \\ \text{P.R. } 5.25 \times 0.80 \times 2.40 \times 2.40 &= 1920 \\ \text{Parade } 0.30 \times 0.15 \times 5.25 \times 1.00 &= 2360 \end{aligned}$$

$$\begin{aligned} \text{Cargas uniformes:} \\ \text{Cabo. } 100 \\ \text{Cabo. } 50 \\ \text{Piso } 80 \\ \text{P.R. } 318 \end{aligned}$$



$$\begin{aligned} \text{7 } \phi 3/8 \text{ P.H.} \\ \text{14 } \phi 3/8 \text{ P.H.} \\ \text{9 } \phi 3/8 \text{ P.H.} \\ \text{9 } \phi 3/8 \text{ P.H.} \end{aligned}$$

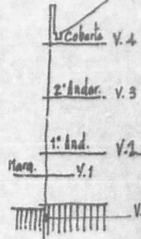
$$\begin{aligned} \text{Cargas uniformes:} \\ \text{Laje } 0.50 \times 0.30 \times 2.15 &= 344 \\ \text{P.R. } 0.50 \times 0.30 \times 2.15 \times 2.40 &= 462 \\ \text{Piso } 0.8 \times 2.15 \times 12.0 &= 1372 \end{aligned}$$

$$\text{Continuação } \sim 1100$$

Pag. 1.

LIVELARIA SELBACH - Cálculos de Cimento Armado da Fachada a ser construída a Rua Nat. Floriano, 10.

Colunas



$$\begin{aligned} \text{3º Per. telhado} \\ \text{Piso } 7.05 \times 1.75 \times 120 &= 1480 \\ \text{Parade } 0.25 \times 3.55 \times 800 &= 1560 \\ \text{Viga } 1.50 \times 3.55 \times 400 &= 2560 \\ \text{P.R. } 0.25 \times 0.25 \times 4.35 \times 800 &= 660 \\ \text{Viga } 3.55 \times 0.30 \times 800 \times 2.40 &= 2050 \end{aligned}$$

$$\begin{aligned} \text{2º And.} \\ \text{Piso } 0.55 \times 3.55 \times 800 &= 1580 \\ \text{Parade } 3.55 \times 1.00 \times 400 &= 1710 \\ \text{Viga } 3.55 \times 0.30 \times 1.00 \times 2.40 &= 2580 \\ \text{P.R. } 0.30 \times 0.30 \times 5.00 \times 2.40 &= 1080 \end{aligned}$$

$$\begin{aligned} \text{1º And.} \\ \text{Piso } 0.55 \times 3.55 \times 800 &= 1580 \\ \text{Parade } 3.55 \times 1.00 \times 400 &= 1710 \\ \text{Viga } 3.55 \times 0.30 \times 1.00 \times 2.40 &= 2580 \\ \text{P.R. } 0.30 \times 0.30 \times 5.00 \times 2.40 &= 1080 \end{aligned}$$

Sapata

$$\frac{26650}{4.5} = 5900$$

$$\frac{30}{77} = 0.39$$

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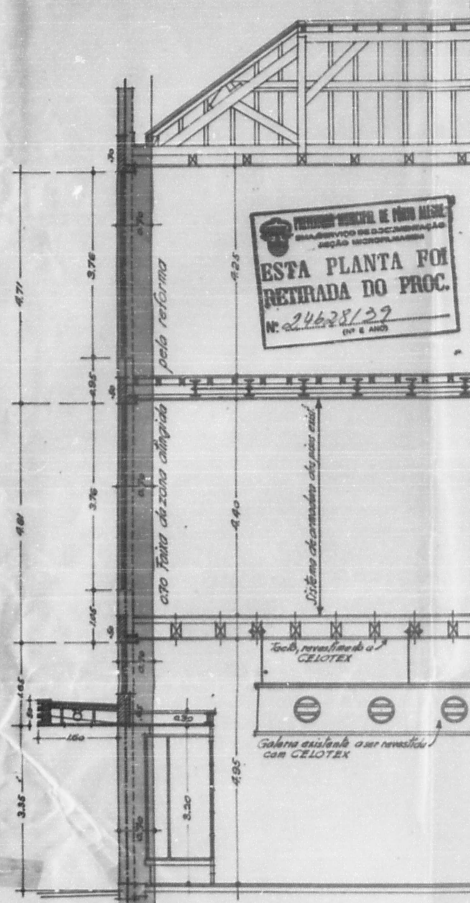
$$\frac{30}{77} = 0.39$$

$$\frac{30}{77} = 0.39$$

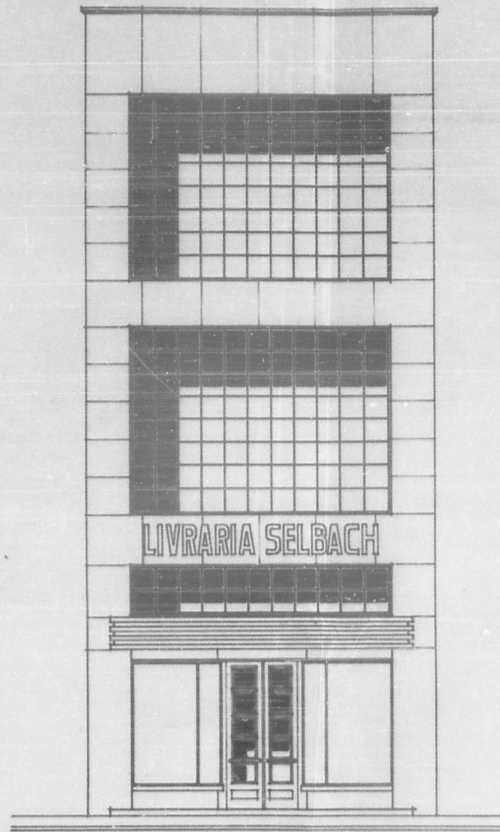
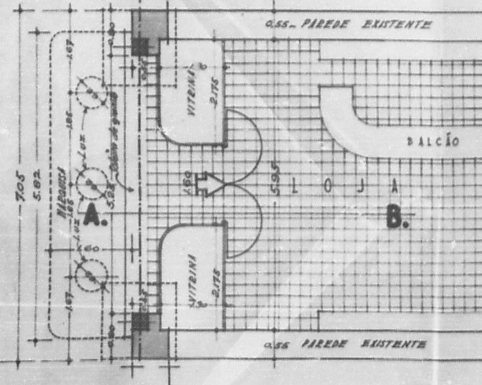
$$\frac{30}{77} = 0.39$$

Continuação.

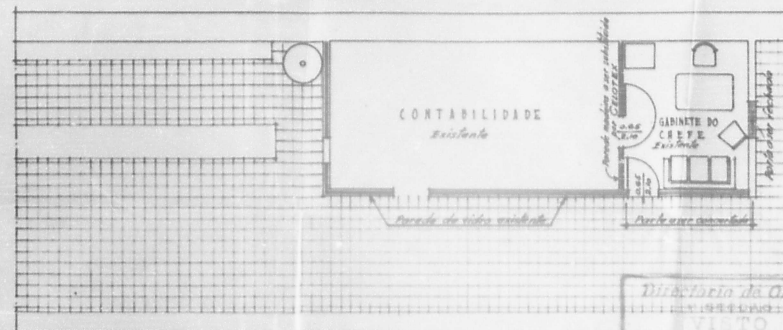
PROJECTO PARA CONSTRUÇÃO DA NOVA FACHADA DO PREDIO Nº 10 DA RUA MARECHAL FLORIANO



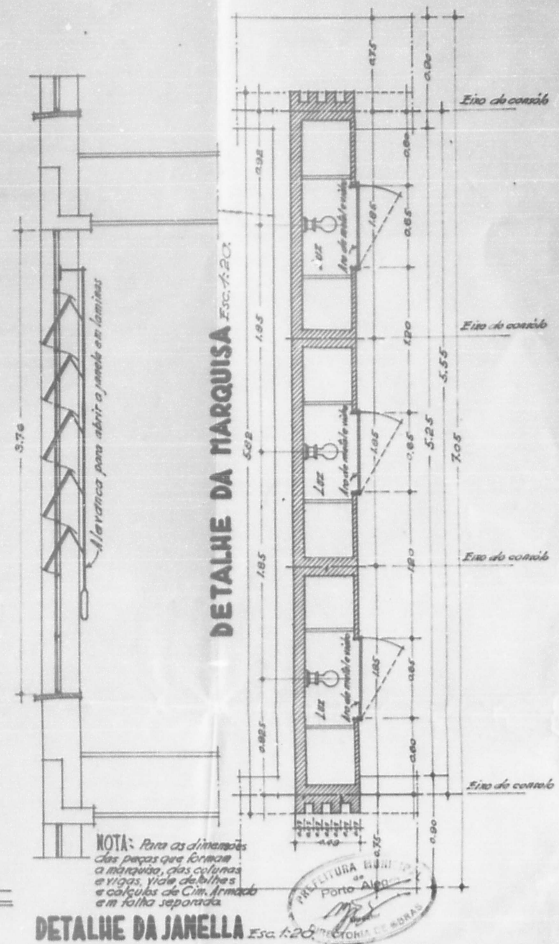
CORTE A-B *ESC. A50*



FACNADA Esc. 450



P L A N T A



DETALHE DA JANELLA

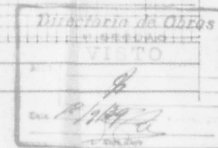
NOTA: Para as dimensões das peças que formam a marquise, das colunas e vigas, vide detalhes e cálculos de Cim. Armada em folha separada.

OS PROPRIETARIOS:



0 CONSTRUCTOR:

Carlos Jung



PROJETO

Amicus

REGISTRADO NO CONSELHO REGIONAL DE
ENGENHARIA E ARQUITETURA, SOB Nº 548

SCHEMATA ESTRUTURAL
Esc. 1:50

FACHADA DA LIVRARIA SELBACH - DETALHES DO CIMENTO ARMADO

Esc. 1:20

ELEVACÃO

Viga: V.4

ESTABELECIDO DE FOMENTO
PARA OBRAS DE RECONSTRUÇÃO
DEPOIS DO INCENDIO
ESTA PLANTA FOI
RETRADA DO PROC.
Nº 24.628/139
DE 1.º AND.

Viga: V.3

7.05

5.25

Viga: V.2

Viga: V.1 (MARQUISA)

Viga: V.0 (MARQUISA)

PLANTA

LIGE. A

LIGE. B

LIGE. C

Fig. T.1

Fig. T.2

Fig. T.3

Fig. T.1

Fig. T.2

Fig. T.3

Fig. T.1

Fig. T.2

Fig. T.3

Fig. T.1

Fig. T.2

Fig. T.3

Fig. T.1

Fig. T.2

Fig. T.3

Fig. T.1

Fig. T.2

Fig. T.3

Fig. T.1

Fig. T.2

Fig. T.3

Fig. T.1

Fig. T.2

Fig. T.3

Fig. T.1

Fig. T.2

Fig. T.3

Fig. T.1

Fig. T.2

Fig. T.3

Fig. T.1

Fig. T.2

Fig. T.3

Fig. T.1

Fig. T.2

Fig. T.3

Fig. T.1

Fig. T.2

Fig. T.3

Fig. T.1

Fig. T.2

Fig. T.3

Fig. T.1

Fig. T.2

Fig. T.3

Fig. T.1

Fig. T.2

Fig. T.3

Fig. T.1

Fig. T.2

Fig. T.3

Fig. T.1

Fig. T.2

Fig. T.3

Fig. T.1

Fig. T.2

Fig. T.3

Fig. T.1

Fig. T.2

Fig. T.3

Fig. T.1

Fig. T.2

Fig. T.3

Fig. T.1

Fig. T.2

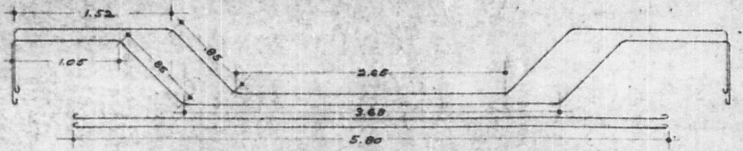
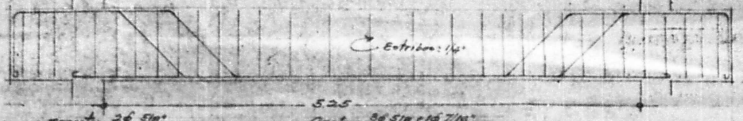
Fig. T.3

Fig. T.1

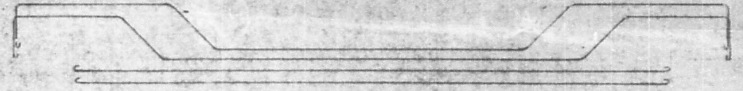
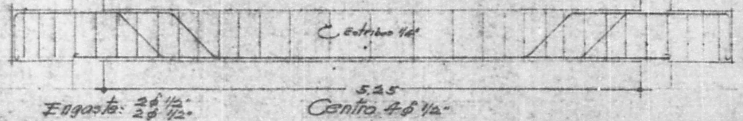
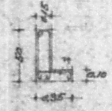
Fig. T.2

Fig. T.3

VIGA: V.4



VIGAS: V.3-V2

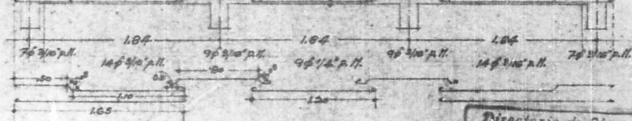


CORTE DA MARQUISA

LIGE. A

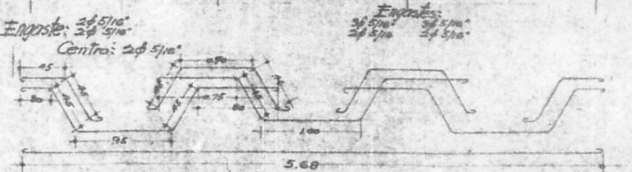
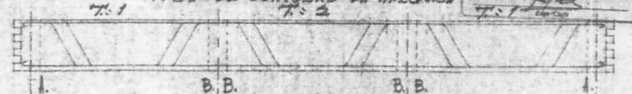
LIGE. B

LIGE. C

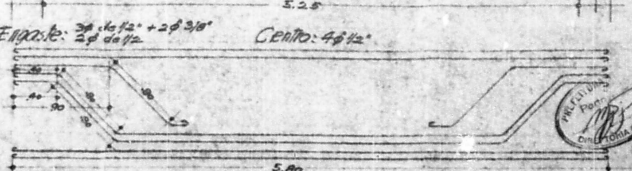
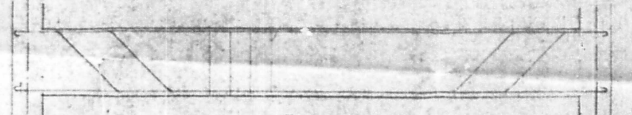


Directorio de Obras
REC. 000
VISTO
T. 1/10/42

VIGA DE CONTEINHO DA MARQUISA



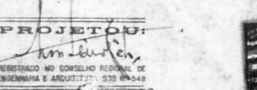
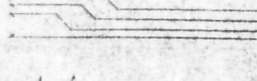
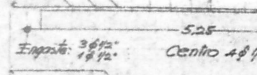
VIGA: V.1 - MARQUISA



5.80

VIGA: V.0 - MARQUISA

DAS COLUNAS



5.80

PROJETO:

Arquiteto

REGRISTRO NO CONSELHO REGIONAL DE

ENGENHARIA E ARQUITETURA - 573 04548

19.1949



Assinado
Carlos J. J. J.

